

Errata

Muni Budhu, "FOUNDATIONS AND EARTH RETAINING STRUCTURES", John Wiley & Sons, 2008

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Page	Line or Fig	Replace	with
83	9	$M = 1.0M = 1.0, R_o = 1.5$	$M = 1.0, R_o = 1.5$
120	16	Uncemented	Cemented
129	2 from bottom	Insert "Undrained" before "Triaxial"	
178	30, 31	0.83	0.94
178	31,35	0.23	0.26
187	Eq. (6.3)	Put end parenthesis ")" on nominator	
189	3 from bottom	2.2	0.47
	1 from bottom	2.2	0.47
	1 from bottom	3.6×10^{-2}	60.6
196	2	Deal	Dead
	10	6.2	6.3
208	9	$r_y =$	$r_q =$
211	21	1×0.45	0.45
212	14	51.3 kN/m	51.3 kN.m
215	2 nd row 3 rd column of table	2.0	2.78
	7 from bottom	34.7	20.4
	9 from bottom	2.0	1.2
	12 from bottom	0.08	0.047
	19 from bottom	34.7 $i_y = 0.08$ kPa/m	20.4 $i_y = 0.047$ kPa
218	Figure E7.6	300 kN/m	300 kN.m
226	Last line	0.92	0.87
227	1	0.92	0.87
		0.15	0.12
		1.6	1.4
228	11	172	176
229	5	σ'_{zc} is the preconsolidation	σ'_{zo} is the current
	12	$\Delta\sigma'_z$	$\Delta\sigma_z$
233	Figure E7.10	7.4	8
239	29	q_u	q_a
240	Table E7.13 Row 6 column F	c_o	l_{co}
246	Last line	FIGURE E7.14	Table E7.14

247	14	k_p	k_h
261	18	$\frac{9}{8}\phi'_{cs}$	$\frac{9}{8}\phi'_p$
263	3 from bottom 4 from bottom	20 s_c	2 s_u
264	19, 20 20	0.45 374	0.8 651
292	18	$s_u^{0.25}$	$s_u^{0.75}$
296	5	Clay:	Step 2: Calculate short term friction capacity of clay
296	7		Insert after line 7 Step 3: Calculate long term friction capacity of clay
299	12	Delete $(OCR)^{1/2}$	
303	2	(Q_p) and $9s_u$	(Q_b) and $9s_u$
303	4	(Q_p)	(Q_b)
304	2	(Q_p)	(Q_b)
310	Figure 8.16(c)	Fine-grained	Coarse-grained
310	Figure 8.16(d)	Fine-grained	Coarse-grained
311	17	Insert "a length of 10 m, " after "using"	
313	10	$I_p = 0.25 + \log(25)$	$Q_{u/t}$
316	7 from bottom	3	4
322	3 after table	eccentricity unit weight of soul	Unit weight of soil
326	3 after table	Remove ")" at end of sentence and place after "semi-empirical"	
327	Line 15 Eq. (8.99) Table 8.6	$Q_{u/t}$ η_1	Q_{ult} η_i
333	Problem 8.9		Add after last line "Assume FS = 2.
334	Figure P8.10	Pile, 0.5 m (1.5 ft)	Pile, 0.6 m (2 ft)
340	24	8.6	16
340	26	1 + 2	1 + 0.2
340	27	s_r	s_γ
340	31	8.6 206.4	16 386.4
340	35	206.4 3	386.4 5.5
341	4	0.785	0.49 (replace both values)
341	4	6.9	4.3 (replace both values)
341	19	10.3.9	103.9
341	24	6.9 + 37.2 = 44.1	4.3 + 37.2 = 41.5
350	24	$B' = B$; $L' =$	$B' = L'$; $B' =$
354	4		Add to the last line. Allowable bearing capacity is 100 kPa (2000 psf).

354	27		After OCR = 4. add Assume these results are representative of the soil at a depth of 12 m (40 ft).
354	35	(a)	(c)
365	Eq. 10.21	<p>Replace</p> $K_{ac} = \frac{\cos^2(\phi' - \eta)}{\cos^2 \eta \cos(\eta + \delta) \left[1 + \left\{ \frac{\sin(\phi' + \delta) \sin(\phi' - \beta)^{\frac{1}{2}}}{\cos(\eta + \delta) \cos(\eta - \beta)} \right\}^2 \right]}$ <p>with</p> $K_{ac} = \frac{\cos^2(\phi' - \eta)}{\cos^2 \eta \cos(\eta + \delta) \left[1 + \left\{ \frac{\sin(\phi' + \delta) \sin(\phi' - \beta)}{\cos(\eta + \delta) \cos(\eta - \beta)} \right\}^{\frac{1}{2}} \right]^2}$	
366	5 and 6 from bottom	$P_{ax} = P_a \cos \delta$ and $P_{px} = P_p \cos \delta$, and the vertical components are $P_{az} = P_a \sin \delta$ and $P_{pz} = P_p \sin \delta$	$P_{ax} = P_a \cos(\delta + \eta)$ and $P_{px} = P_p \cos(\delta + \eta)$, and the vertical components are $P_{az} = P_a \sin(\delta + \eta)$ and $P_{pz} = P_p \sin(\delta + \eta)$
381	11	ϕ'	ϕ_p
382	9	80	180
384	Figure E10.4b	6 1.04	6 kPa 1.4
384	12, 13	K_a	K_{ac}
385	20 20 24 24	656 305.9 656 201.5	653.6 305 653.6 201
391	Eq. (10.78)	$k_h = \rho_d A \left(\frac{A_g \rho_d}{0.087V^2} \right)^{-\frac{1}{4}}$	$k_h = A \left(\frac{Ag \rho_d}{0.087V^2} \right)^{-\frac{1}{4}}$
400	18 from bottom	NPPM	NPPM
401	14	$3K_{ax}(3 + d_o^2)$	$3K_{ax}(3 + d_o)^3$
403	12 from bottom	$\frac{dM_z}{dx}$	$\frac{dM_z}{dz}$
404	Last line	$452.9d^2$	$452.9d$
405	13	265.8	267.4
405	14	265.8 and 116.1	267.4 and 114.5
405	15	116.1 and 232.2	114.5 and 229
408	11 from bottom	in impending fail	at impending failure
410	3 from bottom 2 from bottom	2 x 1.5 1.52	0.2 x 1.5 1.6

422	3 from bottom	Delete "sheet"	
437	9 from bottom	Replace " $\sigma'_b = 20^\circ$ " by " $\phi'_b = 20^\circ$ "	
438	5 14	11.4 1.8	11.9 18
439	10	$(\sigma_x)_{\max}$	$(\sigma_z)_{\max}$
440	8	$L_b = L_b =$	$L_b =$
441	11	(10.77)	(10.78)
452	12 from bottom	Move down "over-consolidated clays" in column 1 to align with "Ladd et al. (1977)"	
454	Table B4.13	Remove shaded areas that are labeled "Gu", "GC", "SU" and "SC"	
455	Table B45.1 First row column 3 after heading	Insert " $C_r = 0.8; L \leq 4 \text{ m (13 ft)}$ "	
456	Table B5.5 Row 2 column 2 after heading	d^2	d^3
457	Table B5.5 Row 13 column 2 after heading	80	60
427 to 429	Add 10. to each problem number. For example 1. becomes 10.1		
447	Add 11. to each problem number. For example 1. becomes 11.1		
462, 463, 464	Replace component in the figure labels with 'component'		